STRUCTURAL ABBREVIATIONS

ACT	ACTUAL
ADDL	ADDITIONAL
ADH	ADHESIVE
ADJ	ADJACENT
AFF	ABOVE FINISHED FLOOR
AHU	AIR HANDLING UNIT
ALUM	ALUMINUM
ALT	ALTERNATE
AHR	ANCHORAGE, ANCHOR
APPD	APPROVED
APPROX	APPROXIMATE, LY
ARCH	ARCHITECT,(URAL)
B–	CONC BEAM
B/	BOTTOM OF
B/B	BACK TO BACK
BITUM	BITUMINOUS, BITUMASTIC
BLDG	BUILDING
BM	BEAM
BOT	BOTTOM
BRDG	BRIDGING
BRG	BEARING
BLK	BLOCK
BRK	BRICK
BTWN	BETWEEN
BSMT	BASEMENT
C	CHANNEL
C–	COLUMN
CANTIL	CANTILEVER
CTR	CENTER, ED
C/C	CENTER TO CENTER
CHFR	CHAMFER
CIRC	CIRCULAR
CJ	CONTROL JOINT
CL	CENTERLINE
CLR	CLEAR
CFS	COLD FORMED STEEL
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONSTR	CONSTRUCTION
CONT	CONTINUOUS, (ATION)
CONTR	CONTRACTOR
COORD	COORDINATE, (TION)
D	DEPTH, DISTANCE
DBA	DEFORMED BAR ANCHOR
DBL	DOUBLE
DBS	DOWEL BAR SPLICER
DEG	DEGREE
DET	DETAIL
DIA	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DL	DEAD LOAD
DN	DOWN
DWGS	DRAWINGS
DWL	DOWEL
E	EAST
EA	EACH
EXP BT	EXPANSION BOLT
EE	EACH END
EF	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION, ELEVATED
ELEV	ELEVATOR
EMBED	EMBEDMENT
ENGR	ENGINEER
EQ	EQUAL
EQUIP	EQUIPMENT
EQUIV	EQUIVALENT
ES	EACH SIDE
EW	EACH WAY
EXP	EXPANSION
EXIST	EXISTING
EXT	EXTERIOR
F–	FOOTING MARK
FC	FILLED CELL
FF	FINISHED FLOOR
FIN	FINISH
FLG	FLANGE
FLR	FLOOR
FDTN	FOUNDATION
FRMG	FRAMING
FS	FARSIDE
FT	FOOT, FEET
FTG	FOOTING
FV	FIELD VERIFY
GA	GAUGE
GALV	GALVANIZED
GB–	GRADE BEAM
GL	GLU LAM
HC	HIGH CHAIR
HK	HOOK
HORIZ	HORIZONTAL
HP	HIGH POINT
HSA	HEADED STUD ANCHOR
HSB	HIGH STRENGTH BOLT
HSS	HOLLOW STRUCTURAL SHAPE
HT	HEIGHT
ID	INSIDE DIAMETER
IF	INSIDE FACE
IN	INCH
INCL	INCLUDE, (ING)
INFO	INFORMATION
INSUL	INSULATE, (D), (ION)
INT	INTERIOR
INTERM	INTERMEDIATE
JG	JOIST GIRDER
JST	JOIST
JT	JOINT
k	KIP
KD	KILN DRY
kg	KILOGRAM
kN	KILONEWTON
L–	ANGLE
LB	POUND
LG	LONG
LL	LIVE LOAD
LLBB	LONG LEG BACK TO BACK
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
LP	LOW POINT
LSL	LONG SLOTTED HOLES
LT	LIGHT
LTWT	LIGHTWEIGHT
LW	LONG WAY
LVL	LAMINATED VENEER LUMBER
M	MEGA, 1000
m	METER
MAS	MASONRY
MAX	MAXIMUM
MB	MACHINE BOLT
MC–	MASONRY COLUMN
MECH	MECHANICAL
MTL	METAL
MEZZ	MEZZANINE
MFR	MANUFACTURER
MID	MIDDLE
MIN	MINIMUM
MISC	MISCELLANEOUS
MJ	MASONRY JOINT
mm	MILLIMETERS
MO	MASONRY OPENING
MPa	MEGAPASCAL
N	NORTH, NEWTON
NIC	NOT IN CONTRACT
NO	NUMBER
NOM	NOMINAL
NS	NEAR SIDE
NTS	NOT TO SCALE
O/O	OUT TO OUT
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OPH	OPPOSITE HAND
OPNG	OPENING
OSB	ORIENTED STRAND BOARD SHEATHING
OSL	OUTSTANDING LEG
OVS	OVERSIZE HOLES
Pa	PASCAL = N/Sq m
PAF	POWDER ACTUATED FASTENER
Pc	PIECE
PC–	PILE CAP
PL	PLATE
PLF	POUNDS PER LINEAL FOOT
PROJ	PROJECTION
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	PRESSURE TREATED
PVC	POLYVINYL CHLORIDE
R/W	REINFORCED WITH
R	RADIUS
REF	REFERENCE
REINF	REINFORCE, (MENT), (ING)
REQ	REQUIRE, (D), (MENT)
RET	RETURN
REV	REVISION
RP	RADIUS POINT
RT	RIGHT
RTU	ROOF TOP UNIT
S	SOUTH
SA	SLEEVE ANCHOR
SB	SLAB BOLSTER
SC	SLIP CRITICAL
SCHED	SCHEDULE
SECT	SECTION
SF–	STEP FOOTING
SIM	SIMILAR
SPEC	SPECIFICATIONS
SP	SPACING, (ES)
SQ	SQUARE
SSL	SHORT SLOTTED HOLES
SST	STAINLESS STEEL
STD	STANDARD
STIF	STIFFENERS
STL	STEEL
STRUCT	STRUCTURAL
SW	SHORT WAY
SYMM	SYMMETRICAL
T	TIES
t	THICKNESS
T/	TOP OF
TB–	TIE BEAM
TC–	TIE COLUMN
TCX	TOP CHORD EXTENSION
T&B	TOP AND BOTTOM
TEMP	TEMPORARY, TEMPERATURE
TERM	TERMINATE
THRD	THREADED
TOL	TOLERANCE
TRAN	TRANSVERSE
TS–	TUBE STEEL
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
U/S	UNDERSIDE
VERT	VERTICAL
W	
W/	WITH
W/O	WITHOUT
WF–	WALL FOOTING
WP	WORKING POINT
WS	WATERSTOP
WT	WEIGHT, STRUCTURAL TEE
WWF	WELDED WIRE FABRIC
WIDTH,	WEST, WIDE FLANGE

STANDARDS

ACI	AMERICAN CONCRETE INSTITUTE
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AITC	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION
AISI	AMERICAN IRON AND STEEL INSTITUTE
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE
APA	AMERICAN PLYWOOD ASSOCIATION
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
AWS	AMERICAN WELDING SOCIETY
CRSI	CONCRETE REINFORCING STEEL INSTITUTE
NFPA	NATIONAL FOREST PRODUCTS ASSOCIATION
PCI	PRESTRESSED CONCRETE INSTITUTE
PTI	POST TENSIONING INSTITUTE
SBC	STANDARD BUILDING CODE
SJI	STEEL JOIST INSTITUTE
UL	UNDERWRITER'S LABORATORY

SYMBOLS

&	AND
@	AT
▶	BEAM MOMENT CONNECTION
℄	CENTERLINE
℉	CONSTRUCTION JT.
ø	DIAMETER
°	DEGREE
◆	ELEVATION
d	NAIL (PENNY WEIGHT)
#	NUMBER, POUNDS
▬	MASONRY FILLED CELL
℔	PLATE
⅋	SAW CUT JOINT

STRUCTURAL DESIGN CRITERIA

D-1 CODES: – FLORIDA BUILDING CODE 2010 EDITION
ASCE 7–10

D-2 DESIGN LIVE LOADS: ROOF 20 PSF

D-3 DESIGN WIND SPEED (ULTIMATE): 136 MPH
RISK CATEGORY II
ENCLOSURE CLASSIFICATION: PARTIALLY ENCLOSED
EXPOSURE CATEGORY C
INTERNAL PRESSURE COp: ±0.55
COMPONENTS & CLADDING DESIGN WIND PRESSURE: SEE SCHEDULE

D-4 ALLOWABLE SOIL BEARING PRESSURE 2500 PSF, BASED ON REPORT MADE BY NODARSE DATED NOVEMBER 28, 2012.

GENERAL NOTES

G-1 REVIEW ALL PROJECT DOCUMENTS PRIOR TO FABRICATION AND START OF CONSTRUCTION. REPORT ANY DISCREPANCIES TO ARCHITECT OR STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH WORK. DIMENSIONS RELATING TO EXISTING CONSTRUCTION ARE TO BE FIELD VERIFIED.

G-2 THE STRUCTURE IS DESIGNED TO BE VIABLE IN ITS FINAL, FULLY ERECTED CONFIGURATION IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO ENSURE THE SAFETY AND STABILITY OF THE BUILDING AND ITS COMPONENT PARTS DURING THE CONSTRUCTION PHASE OF THE PROJECT.

G-3 THE EXTENT OF UNDERGROUND UTILITIES IS UNKNOWN. IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROTECT ALL EXISTING CONSTRUCTION DURING EXCAVATION.

G-4 NO STRUCTURAL MEMBER SHALL BE CUT, NOTCHED OR OTHERWISE REDUCED IN SIZE OR STRENGTH WITHOUT PRIOR APPROVAL IN WRITING FROM THE STRUCTURAL ENGINEER.

G-5 COORDINATE STRUCTURAL AND OTHER DRAWINGS THAT ARE PART OF THE CONTRACT DOCUMENTS FOR ANCHORED, EMBEDDED OR SUPPORTED ITEMS WHICH MAY AFFECT THE STRUCTURAL DRAWINGS.

G-6 ALL DETAILS AND SECTIONS ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE ON THE PROJECT EXCEPT WHERE A SEPARATE DETAIL IS SHOWN.

G-7 THE INTENTION OF THE PLANS AND SPECIFICATIONS IS TO PROVIDE ALL NECESSARY DETAILS TO CONSTRUCT A COMPLETE STRUCTURE. WHEN SPECIFIC INFORMATION IS MISSING OR IS IN CONFLICT, THE CONTRACTOR SHALL USE A SIMILAR DETAIL AND/OR THE MORE COSTLY ITEM OF CONFLICT. THE CONFLICTS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER.

G-8 THE ENGINEER SHALL NOT BE RESPONSIBLE FOR LAYOUT, DIMENSIONAL ERRORS OR DISCREPANCIES RESULTING FROM THE REPRODUCTION AND USE OF CONTRACT DRAWINGS FOR ERECTION AND SHOP DRAWINGS. USE OF CONTRACT DRAWINGS REPRODUCED IN WHOLE OR IN PART IN SHOP DRAWINGS SHALL NOT RELIEVE THE CONTRACTOR NOR SUBCONTRACTORS FROM THEIR RESPONSIBILITY TO ACCURATELY LAYOUT, COORDINATE, DETAIL, FABRICATE AND INSTALL A COMPLETE STRUCTURE.

G-9 REVIEW ALL SHOP DRAWINGS FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS AND FOR COMPLETENESS AND ANSWER ALL CONTRACTOR RELATED QUESTIONS. STAMP AND INITIAL ALL SHEETS PRIOR TO SUBMITTING SHOP DRAWINGS TO ENGINEER FOR REVIEW. NON-COMPLIANCE WITH THIS REQUIREMENT SHALL RESULT IN REJECTION OF SUBMITTAL.

G-10 SHOP DRAWING SUBMITTALS SHALL BE CONSISTENT WITH THE SPECIFICATIONS. ADDITIONAL SETS OF SHOP DRAWINGS OTHER THAN THOSE REQUIRED WILL BE DISCARDED.

SHALLOW FOUNDATIONS

SF-1 SOIL SHALL BE STRIPPED, COMPACTED AND TESTED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT AND PROJECT SPECIFICATIONS.

SF-2 CENTER ALL FOOTINGS UNDER THEIR RESPECTIVE COLUMNS OR WALLS UNLESS OTHERWISE SHOWN ON PLANS. MAXIMUM MISPLACEMENT OR ECCENTRICITY – 2". TOLERANCE FOR MISLOCATION OF COLUMN DOWELS OR ANCHOR BOLTS TO BE PER ACI OR AISC STANDARDS.

SF-3 HORIZONTAL JOINTS IN FOOTINGS WILL NOT BE PERMITTED.

SF-4 WHERE VERTICAL CONSTRUCTION JOINTS OCCUR IN CONTINUOUS FOOTINGS, PROVIDE A MINIMUM OF ONE CONTINUOUS 2" x 4" KEYWAY ACROSS JOINT FOR EACH 12" OF DEPTH.

SF-5 CONTRACTOR TO NOTIFY ENGINEER IF SOIL CONDITIONS ARE UNCOVERED THAT PREVENT THE REQUIRED SOIL BEARING PRESSURE FROM BEING OBTAINED.

SF-6 COORDINATE PLUMBING LINES WITH FOOTING LOCATIONS FOR INTERFERENCE. INDIVIDUAL FOOTINGS SHALL BE LOWERED WITH THE PRIOR APPROVAL OF THE STRUCTURAL ENGINEER. CONTINUOUS WALL FOOTINGS SHALL BE STEPPED AS DETAILED ON THE DRAWINGS.

SF-7 EXCAVATING UNDER OR NEAR IN-PLACE FOOTINGS/FOUNDATIONS WHICH DISTURBS THE COMPACTED SOIL BENEATH THE FOOTINGS/ FOUNDATIONS SHALL NOT BE PERMITTED.

SF-8 REINFORCING SHALL BE SUPPORTED ON PRECAST CONCRETE PADS. DOWELS FOR COLUMNS AND FILLED CELLS SHALL BE SECURED IN PLACE PRIOR TO PLACING CONCRETE. USE TEMPLATES FOR SETTING COLUMN DOWELS AND ANCHOR BOLTS.

REINFORCED CONCRETE

RC-1 ALL CONCRETE DESIGN AND PLACEMENT SHALL BE IN STRICT ACCORDANCE WITH THE ACI "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE," ACI 318.

RC-2 STRUCTURAL CONCRETE SHALL CONFORM TO ACI 301 SPECIFICATIONS AND SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS.

RC-3 USE REGULAR WEIGHT CONCRETE. SUBMIT DESIGN MIX FOR REVIEW PRIOR TO PLACEMENT OF CONCRETE. ALL MIX DESIGNS SHALL CLEARLY INDICATE INTENDED USE AND LOCATION.

RC-4 STRUCTURAL CONCRETE SHALL HAVE #57 COURSE AGGREGATE WITH A SLUMP RANGE OF 3' TO 5'. HIGHER SLUMPS ARE ACCEPTABLE WITH THE ADDITION OF A HIGH RANGE WATER REDUCING ADMIXTURE (SUPERPLASTICIZER).

RC-5 FLYASH, WHEN USED, SHALL BE LIMITED TO 20% OF THE CEMENTITIOUS MATERIAL.

RC-6 ALL PUMPED CONCRETE WITH #57 AGGREGATE IS TO CONTAIN A HIGH RANGE WATER REDUCING AGENT. MINIMUM SIZE OF DISCHARGE TO BE 4" I.D.

RC-7 A 2" I.D. DISCHARGE MAY BE USED WITH #89 AGGREGATE. USE HIGH RANGE WATER REDUCING ADMIXTURE IF NECESSARY TO INCREASE SLUMPS BEYOND THAT NOTED ABOVE.

RC-8 CHAMFER ALL EDGES OF EXPOSED CONCRETE 3/4" UNLESS NOTED OTHERWISE.

RC-9 ALL REINFORCING STEEL SHALL BE DETAILED, FABRICATED AND INSTALLED IN ACCORDANCE WITH ACI 318 AND ACI DETAILING MANUAL, ACI-315 MOST CURRENT EDITIONS.

RC-10 REINFORCING STEEL SHALL BE NEW DEFORMED BARS THAT ARE FREE FROM RUST, SCALE AND OIL AND CONFORM TO ASTM A615, GRADE 60, WITH MINIMUM YIELD STRENGTH = 60,000 PSI.

RC-11 SHOP DRAWINGS FOR PLACEMENT SHALL BE SUBMITTED FOR REVIEW PRIOR TO REBAR FABRICATION.

RC-12 WELDED WIRE FABRIC SHALL CONFORM TO ASTM-A185.

RC-13 LAP CONTINUOUS REINF. AS NOTED IN LAP SPLICE SCHEDULE. LAP CONT. BOTTOM STEEL OVER SUPPORT AND CONT. TOP STEEL AT MIDSPAN UNLESS OTHERWISE SPECIFIED.

RC-14 TERMINATE ALL DISCONTINUOUS TOP BARS WITH STANDARD 90 DEGREE HOOK (PLACED VERTICALLY) UNLESS NOTED OTHERWISE.

RC-15 AT CHANGES IN DIRECTION OF CONCRETE WALLS, BEAMS & STRIP FOOTINGS, PROVIDE CORNER BARS OF SAME SIZE AND QUANTITY (U.N.O.) AS HORIZONTAL STEEL. REFER TO TYPICAL DETAIL.

RC-16 PROVIDE THE FOLLOWING CONCRETE COVERAGES OVER REINFORCING (U.N.O.):

FOOTINGS:	BOTTOM AND SIDES	3" CLR.
	TOP	2" CLR.
BEAMS & COLUMNS (TO TIES):	INTERIOR	1 1/2" CLR.
CONC. WALLS:		2" CLR.
SLABS ON GRADE:	SINGLE MAT	AS SHOWN

RC-17 FOOTING SIZES SHOWN ARE FOR FOOTINGS CONSTRUCTED WITH SIDE FORMS. IF SOIL MATERIAL CAN HOLD A VERTICAL SHAPE, THE SOIL CAN BE USED AS AN EARTH FORM PROVIDED OVERALL FOOTING WIDTH IS INCREASED 4", AND CODE COVER INCREASED BY 2". ALL SLOUGHED MATERIAL SHALL BE REMOVED FROM EXCAVATION BEFORE AND DURING PLACEMENT OF CONCRETE.

RC-18 USE THE STRUCTURAL DRAWINGS, INCLUDING REVISIONS AND ADDENDA, IN CONJUNCTION WITH REVIEWED SHOP DRAWINGS FOR PLACEMENT OF REINFORCING.

RC-19 SLEEVE ALL PENETRATIONS THROUGH BEAMS AND SLABS INDIVIDUALLY. CORE DRILLING SHALL NOT BE PERMITTED. SUBMIT LOCATION AND SIZE OF SLEEVES THROUGH BEAMS TO ENGINEER FOR REVIEW PRIOR TO CASTING CONCRETE.

RC-20 NO REINFORCING BARS SHALL BE CUT TO ACCOMMODATE THE INSTALLATION OF ANCHORS, EMBEDS OR OTHER ITEMS.

RC-21 ALL EMBEDDED ITEMS SHALL BE SECURELY TIED IN PLACE PRIOR TO CONCRETE PLACEMENT.

RC-22 PLACEMENT OF CONDUIT AND PIPES IN CONCRETE SHALL CONFORM TO ACI 318 (6.3).

RC-23 PLACE CONCRETE PER ACI 304. USE INTERNAL MECHANICAL VIBRATION FOR ALL CONCRETE. LIMIT MAXIMUM FREE FALL DROP OF COLUMN OR WALL CONCRETE TO 6'-0" FOR #57 AGGREGATE AND 8'-0" FOR #89 AGGREGATE. IN SLABS, BEAMS, FOOTINGS, ETC., LIMIT FREE FALL DROP TO 3'-0". ALL PRECAUTIONS SHALL BE TAKEN TO AVOID SEGREGATION OF CONCRETE DURING PLACEMENT.

RC-24 WEDGE BOLTS SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST ACI 355.2 AND PROVIDE THE FOLLOWING ALLOWABLE LOADS FOR 4 3/4" EMBEDMENT DEPTH IN 3000 PSI CONCRETE:

TENSION 4.2K
SHEAR 5.3K

INSTALLATION SHALL BE BY QUALIFIED PERSONNEL AND INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII).

RC-25 ADHESIVE ANCHORS SHALL CONFORM TO THE LATEST ACI 355.4 AND PROVIDE THE FOLLOWING MINIMUM STRENGTHS FOR 3/4@ A307 THREADED ROD: UNCRACKED CONCRETE BOND STRENGTH OF 1385 PSI AND CRACKED CONCRETE BOND STRENGTH OF 710 PSI. INSTALLATION SHALL BE BY QUALIFIED PERSONNEL AND INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII).

RC-26 HEADED STUD ANCHORS AND STUD WELDING SHALL CONFORM TO AWS D1.1 STRUCTURAL WELDING CODE', TYPE B MADE FROM ASTM A108, AND HAVE A MINIMUM TENSILE STRENGTH OF 65KSI.

RC-27 DOWEL BAR SPLICERS SHALL BE A COMPLETE ASSEMBLY, INCLUDING COUPLER, FORM ATTACHMENT (WHEN REQUIRED) AND THREADED BAR CAPABLE OF DEVELOPING THE BAR BEING SPLICED IN ACCORDANCE WITH ACI 318 (12.14.3).

12047
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WINTER PARK, FLORIDA 32789
EB #767

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CONSTRUCTION DOCUMENTS - MARCH 10, 2014

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FLORIDA REGISTRATION NUMBERS
#AA-C000148 / # IB-0000809

GENERAL NOTES

THEODORE G. FERY
FLORIDA REGISTRATION AR0007939

KEYED NOTES

STEVEN L. SHELT, PE 0036171

	OWNER COMMENTS	01/16/2014
-	-	---/--/--

NO.: DESCRIPTION: DATE:

ORANGE COUNTY STORM-WATER MANAGEMENT BUILDING 5A

4200 JOHN YOUNG PKWY
ORLANDO, FL 32839

LIST OF BOARD MEMBERS:

ORANGE COUNTY MAYOR	TERESA JACOBS
DISTRICT 1 COMMISSIONER	S. SCOTT BOYD
DISTRICT 2 COMMISSIONER	FRED BRUMMER
DISTRICT 3 COMMISSIONER	PETE CLARKE
DISTRICT 4 COMMISSIONER	JENNIFER THOMPSON
DISTRICT 5 COMMISSIONER	TED EDWARDS
DISTRICT 6 COMMISSIONER	TIFFANY MOORE RUSSELL

STRUCTURAL NOTES & ABBREVIATIONS

DRAWN BY:	RBJ
CHECKED BY:	KCB
SCALE:	1/8"
START DATE:	06.12.2013
PROJECT NO.:	3764A-00

SHEET NO.: S-001

FILE: \ACBIGBRO\Drawings\DWGS\12047\ 12047-S-001 - Bldg 5a.dwg
Plot Date: 3/4/2014 6:46:38 AM

MASONRY

M-1 MASONRY CONSTRUCTION SHALL CONFORM TO ACI "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530-08/ASCE 5-08) AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1-08/ASCE 6-08), EXCEPT AS AMENDED BELOW.

M-2 CONTRACTOR SHALL OBTAIN COPY OF MASONRY CODE AND SPECIFICATIONS FOR REFERENCE AT THE JOBSITE.

M-3 STRUCTURE HAS BEEN DESIGNED AS A BEARING WALL STRUCTURE. ALL MASONRY UNITS SHALL BE LAID PRIOR TO CONCRETE PLACEMENT OF COLUMNS, BEAMS AND SLABS FOR THE SAME STORY.

M-4 USE TYPE "S" MORTAR WITH MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI.

M-5 MASONRY UNITS SHALL BE LAID IN RUNNING BOND AND (UNO) CONFORM TO ASTM C90 WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI ON NET SECTION TO PROVIDE NET AREA COMPRESSIVE STRENGTH OF MASONRY (F'm) OF 1500 PSI.

M-6 PROVIDE FILLED CELLS AS SHOWN ON PLANS. IN ADDITION, PROVIDE FILLED CELLS ADJACENT TO ALL OPENINGS AND AT ANCHORAGE OF CONNECTIONS. GROUT SHALL BE IN ACCORDANCE WITH ASTM C476, HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI AT 28 DAYS AND BE TESTED IN ACCORDANCE WITH ASTM C1019. GROUT SLUMP SHALL BE BETWEEN 8 AND 11". THE MAXIMUM AGGREGATE SIZE SHALL BE #89 PEAROCK.

M-7 USE METAL LATH OR CAVITY CAPS BELOW TIE BEAMS OR KNOCK-OUT COURSES TO BLOCK UNFILLED CELLS. FELT OR OTHER BOND BREAKING MATERIAL SHALL NOT BE USED.

M-8 PROVIDE FULL MORTAR BEDDING AROUND ALL FILLED CELLS WITH VERTICAL REINFORCING.

M-9 REINFORCING FOR FILLED CELLS SHALL CONFORM TO ASTM A615, GRADE 60. PROVIDE THE FOLLOWING LAP SPLICES FOR REINFORCING:

#4 BARS	26"
#5 BARS	32"
#6 BARS	58"

M-10 ALL FILLED CELLS AND COLUMNS SHALL BE POURED AT LEAST TWO (2) HOURS PRIOR TO POURING BEAMS AND LINTELS.

M-11 REINFORCE WALL WITH LADDER TYPE REINFORCEMENT ("DUR-O-WALL" OR EQUAL) IN BED JOINTS AT 16" O.C. MEASURED VERTICALLY. LAP SPLICE ALL HORIZONTAL WALL REINFORCING 6". PROVIDE PREFABRICATED "TEE" OR CORNER SECTIONS AT ALL INTERSECTING WALLS.

M-12 REFER TO TYPICAL WALL SECTIONS FOR MAXIMUM CONSTRUCTION HEIGHT OF MASONRY WALLS. PROVIDE CLEAN-OUT HOLES AT BASE OF FILLED CELL WHEN THE CONCRETE POUR EXCEEDS 5 FEET IN HEIGHT.

M-13 CONCRETE FOR FILLED CELLS SHALL BE VIBRATED DURING PLACEMENT USING A "PENCIL" TYPE VIBRATOR.

M-14 UNLESS NOTED OTHERWISE, PROVIDE THE FOLLOWING LINTELS OVER ALL MASONRY WALL OPENINGS:

OPENING LESS THAN OR EQUAL TO 6'-0": 8" DEEP PRECAST "U" LINTEL WITH 1-#5 AND FILLED SOLID WITH CONCRETE.

GREATER THAN 6'-0" TO LESS THAN OR EQUAL TO 10'-0": 8" DEEP KNOCK-OUT LINTEL COURSE AND 8" DEEP PRECAST "U" LINTEL. PROVIDE 1-#5 IN EACH COURSE AND FILLED SOLID WITH CONCRETE.

PROVIDE MINIMUM END BEARING OF 8". CUT OUT BOTTOM OF LINTEL AT END TO ALLOW CONTINUATION OF FILLED CELL REINFORCING.

M-15 UNLESS NOTED OTHERWISE, EXTEND ALL NON-BEARING MASONRY WALLS MINIMUM 8" ABOVE CEILING. CAP WALL WITH KNOCK-OUT LINTEL COURSE. REINFORCE WITH 1-#5 CONT. AND FILL SOLID WITH CONCRETE.

M-16 PROVIDE #5 VERTICAL IN CONCRETE FILLED CELLS @ 4'-0" O.C. MAX AND #5 CONTINUOUS HORIZONTAL IN KNOCK-OUT BLOCK BELOW WINDOW SILL UNLESS NOTED OTHERWISE.

M-17 PROVIDE A SINGLE CORNER BAR FOR EACH MASONRY TIE BEAM OR BOND BEAM COURSE THAT CHANGES DIRECTION OR INTERSECTS OTHER CONCRETE OR MASONRY BEAMS. BAR SIZE TO BE SAME AS LARGEST HORIZONTAL BAR. SEE MASONRY LAP SCHEDULE FOR LAP REQUIREMENTS.

STRUCTURAL STEEL

S-1 FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AISC "MANUAL OF STEEL CONSTRUCTION," THIRTEENTH EDITION AND THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS," 2005 EDITION.

S-2 ALL STRUCTURAL 'W' SHAPES TO CONFORM TO ASTM A992 WITH MINIMUM YIELD STRENGTH OF 50 KSI. ANGLES, CHANNELS AND PLATES SHALL CONFORM TO ASTM A36 WITH A MINIMUM YIELD STRENGTH OF 36 KSI.

SQUARE AND RECTANGULAR STEEL TUBES (HSS SECTIONS) SHALL CONFORM TO ASTM A500, GRADE B, WITH A MINIMUM YIELD STRENGTH OF 46 KSI.

S-3 ALL HIGH-STRENGTH BOLTS SHALL MEET THE REQUIREMENTS OF THE "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."

S-4 UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE 3/4" DIAMETER A325 AND SHALL BE BEARING TYPE CONNECTIONS.

S-5 USE MIN. 3/4" A325F BOLTS WITH CLASS A CONTACT SURFACE FOR SLIP CRITICAL CONNECTIONS AS DESIGNATED ON PLANS BY A325 S.C.

S-6 ALL ANCHOR RODS CAST IN CONCRETE SHALL CONFORM TO ASTM F1554 GRADE 36.

S-7 ALL SHOP AND FIELD WELDING SHALL BE DONE BY CURRENTLY QUALIFIED WELDERS IN ACCORDANCE WITH AWS D1.1 "STRUCTURAL WELDING CODE," LATEST EDITION.

S-8 USE E70XX LOW HYDROGEN ELECTRODES FOR ALL WELDING UNLESS NOTED OTHERWISE. GRIND SMOOTH ALL EXPOSED WELDS.

S-9 SUBMIT STRUCTURAL STEEL SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. CLEARLY SHOW ALL PIECE MARKS, CONNECTIONS AND ERECTION DRAWINGS. ANY SPLICES NOT SHOWN ON CONTRACT DRAWINGS ARE TO BE CLEARLY NOTED FOR APPROVAL.

S-10 DO NOT WELD TO EMBEDS UNTIL CONCRETE HAS CURED AT LEAST 72 HOURS. USE APPROPRIATE WELDING PROCESSES TO LIMIT HEAT BUILDUP IN EMBED TO AVOID PLATE EXPANSION AND CRACKING OF CONCRETE.

S-11 PROVIDE 1 1/2 MIL THICKNESS OF COLD GALVANIZED COATING TO ALL STEEL SURFACES BELOW GRADE. COATING SHALL CONTAIN MINIMUM 95% METALLIC ZINC BY WEIGHT IN DRY FILM. (ASTM D520 TYPE III). VERIFY COMPATIBILITY WITH FINISH COAT AND/OR FIREPROOFING.

S-12 ALL EXTERIOR EQUIPMENT CURBS, MECHANCICAL EQUIPMENT TIE DOWNS AND CONNECTIONS TO THE BUILDING STRUCTURE ARE TO HAVE SIGNED AND SEALED CALCULATIONS AND DRAWINGS SUBMITTED FOR REVIEW.

STEEL JOISTS

SJ-1 ALL DESIGN, FABRICATION AND ERECTION OF STEEL JOISTS AND BRIDGING SHALL BE IN STRICT ACCORDANCE WITH THE CURRENT SPECIFICATIONS OF THE STEEL JOIST INSTITUTE.

SJ-2 DO NOT INSTALL JOISTS UNTIL SUPPORTING CONCRETE ELEMENTS HAVE CURED AT LEAST 48 HOURS.

SJ-3 THE BOTTOM CHORD OF ALL JOISTS SHALL BE CONSTRUCTED OF DOUBLE ANGLES.

SJ-4 REFER TO SPECIFIC DETAILS FOR LENGTH OF BEARING ENDS AND BOTTOM CHORD EXTENSIONS.

SJ-5 PROVIDE CONTINUOUS HORIZONTAL TOP AND BOTTOM CHORD BRIDGING AND "X" BRIDGING PER SJI STANDARDS AND AS SHOWN ON PLANS.

SJ-6 ANCHOR THE ENDS OF ALL BRIDGING WHICH TERMINATES AT WALLS OR BEAMS PER STRUCTURAL DETAILS.

SJ-7 DO NOT APPLY CONCENTRATED LOADS FOR PIPING, MECHANICAL UNITS, ETC., EXCEEDING 200 POUNDS BETWEEN JOIST PANEL POINTS WITHOUT PROVIDING REINFORCEMENT PER APPLICABLE DETAIL.

SJ-8 FABRICATOR TO DESIGN JOISTS AND SUPPLY ADDITIONAL BRIDGING AS REQUIRED TO RESIST A NET UPLIFT FORCE AS FOLLOWS:

INTERIOR ROOF SPAN 24 PSF

PROVIDE SIGNED AND SEALED CALCULATIONS AS REQUIRED TO VERIFY INTENT OF DESIGN CRITERIA FOR UPLIFT, 'SPECIAL' JOISTS AND OTHER LOAD REQUIREMENTS NOTED IN PROJECT DOCUMENTS.

SJ-9 JOIST SHOP DRAWINGS MAY BE REVIEWED FOR GENERAL CONFORMANCE WITH THE PROJECT DRAWINGS AND REQUIREMENTS WITHOUT SUPPORTING CALCULATIONS. IN THAT EVENT, THE SHOP DRAWINGS WILL BE REVIEWED TO THE EXTENT POSSIBLE AND REJECTED. FINAL REVIEW REQUIRES SUBMISSION OF DRAWINGS AND SUPPORTING CALCULATIONS CONCURRENTLY.

METAL DECK

MD-1 METAL ROOF DECK SHALL BE AS FOLLOWS:

15/16" CORRUGATED METAL DECK, 26GA, GALVANIZED

MD-2 INSTALL METAL DECK IN ACCORDANCE WITH REQUIREMENTS OF MANUFACTURER AND STEEL DECK INSTITUTE, INCLUDING TEMPORARY SHORING. PROVIDE SHORING WHENEVER THE SPAN EXCEEDS SDI MAXIMUM UNSHORED CLEAR SPAN AND FOR CANTILEVERED CONDITIONS AS NOTED IN PLANS AND DETAILS.

MD-3 ATTACH METAL DECK TO SUPPORTS TO PROVIDE DIAPHRAGM AS FOLLOWS:

ROOF DECK
WELD DECK WITH (30/9) 5/8" PUDDLE WELDS AT ALL STEEL SUPPORTS. PERIMETER WELDING SHALL HAVE A MAXIMUM SPACING OF 6" O.C. USE WELD WASHERS FOR ALL DECK LESS THAN 22 GA. ATTACH SIDE LAPS WITH #12 SCREWS AT 2'-0" O.C.

MD-4 UNLESS NOTED OTHERWISE ON PLAN, SUPPORT ALL PERIMETER EDGES OF METAL DECK WITH CONTINUOUS L3 x 3 x 1/4. WELD TO STRUCTURAL STEEL EMBED PLATES AT 4'-0" O.C. MAXIMUM OR ATTACH TO CONCRETE OR FILLED MASONRY BLOCK CELL WITH 3/4" WEDGE BOLTS AT 4'-0" O.C. MAXIMUM. LOCATE END SUPPORT ATTACHMENT 0'-8" FROM ENDS.

MD-5 ALL DECK SUPPORT ANGLE SPLICES INCLUDING CORNERS ARE TO OCCUR OVER SUPPORTS AND EACH LEG IS TO BE FULLY WELDED.

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FLORIDA REGISTRATION NUMBERS
#AA-C000148 / # IB-0000809

GENERAL NOTES

THEODORE G. FERY
FLORIDA REGISTRATION AR0007939

KEYED NOTES

STEVEN L. SHELT, PE 0036171

NO.:	DESCRIPTION:	DATE:
-	-	----/--/--

ORANGE COUNTY STORM-WATER MANAGEMENT BUILDING 5A

4200 JOHN YOUNG PKWY
ORLANDO, FL 32839

LIST OF BOARD MEMBERS:

ORANGE COUNTY MAYOR	TERESA JACOBS
DISTRICT 1 COMMISSIONER	S. SCOTT BOYD
DISTRICT 2 COMMISSIONER	FRED BRUMMER
DISTRICT 3 COMMISSIONER	PETE CLARKE
DISTRICT 4 COMMISSIONER	JENNIFER THOMPSON
DISTRICT 5 COMMISSIONER	TED EDWARDS
DISTRICT 6 COMMISSIONER	TIFFANY MOORE RUSSELL

STRUCTURAL NOTES CONTINUED

DRAWN BY:	RBJ
CHECKED BY:	KCB
SCALE:	1/8"
START DATE:	06.12.2013
PROJECT NO.:	3764A-00
SHEET NO.:	S-002

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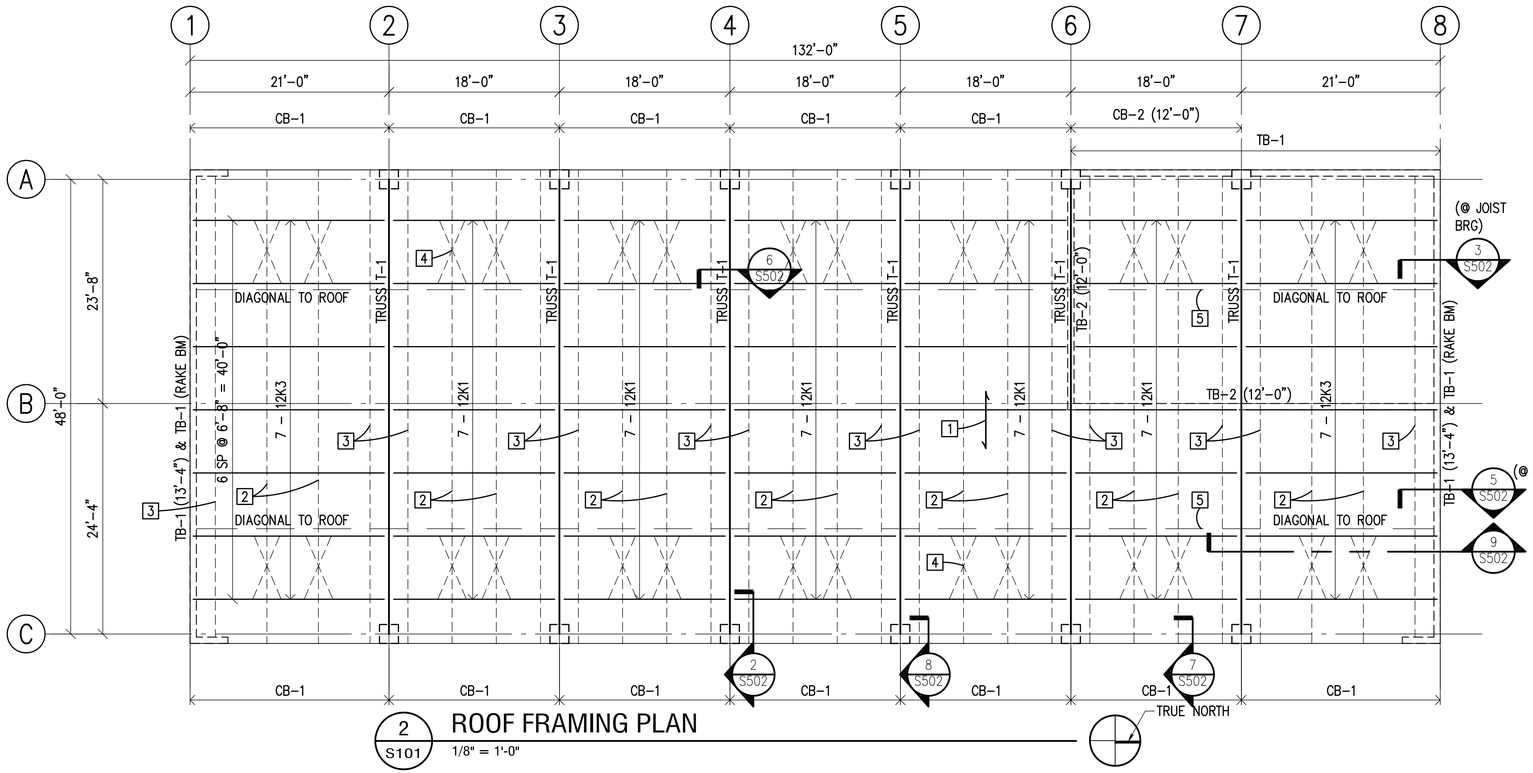
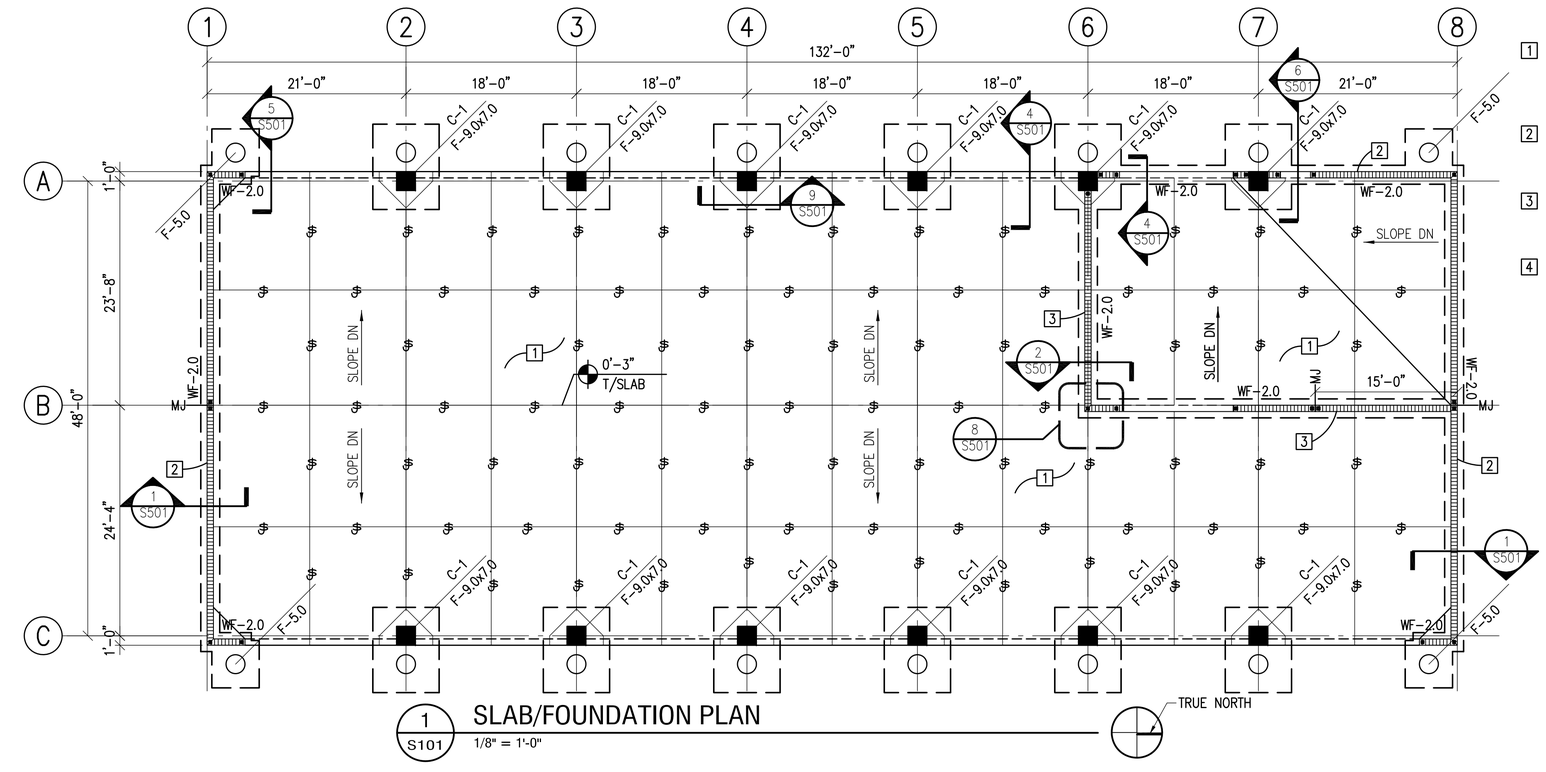


2 S101 **ROOF FRAMING PLAN**

1/8" = 1'-0"

FLOOR FRAMING NOTES:

1. 15/16" CORRUGATED GALVANIZED METAL DECKING. ⟵ INDICATES SPAN DIRECTION. PROVIDE (9) 5/8" PUDDLE WELDS PER 30" SHEET TO ALL TRANSVERSE SUPPORTS. PROVIDE 5/8" PUDDLE WELDS AT 1'-0" OC AT SIDE PERIMETER SUPPORTS. JOIN SIDE LAPS WITH #12 TEK SCREWS AT 2'-0" OC.
2. TYP TOP & BOT CHORD CONT JST BRDG L-1 1/2x1 1/2x7/64.
3. JST UPLIFT BRIDGING L-1 1/2x1 1/2x7/64 TYPICAL AT BOTTOM CHORD.
4. PROVIDE DIAGONAL CROSS BRIDGING AS SHOWN, L-1 1/4x 1 1/4x7/64 MINIMUM.
5. BOTTOM CHORD CROSS BRACING: HSS-4x4x1/4.



1 S101 **SLAB/FOUNDATION PLAN**

1/8" = 1'-0"

FOUNDATION NOTES:

1. 6" (ACTUAL DEPTH) CONC. SLAB REINF w/ #4 @ 12" OC EW CENTERED IN SLAB ON VAPOR RETARDER OVER COMPACTED SUBGRADE. COORD SLOPES, RECESSES, FINISHES, JOINTS, ETC. w/ ARCH DWGS.
2. 8" CMU WALL REINF w/ #6 VERT & MATCHING DWL IN GROUT FILLED CELL @ 24" O.C. & AT CORNERS, INTERSECTIONS & EACH SIDE OF WALL OPENINGS.
3. 8" CMU WALL REINF WITH #5 VERT AND MATCHING DOWEL IN GROUT FILLED CELL AT 48" OC AND AT CORNERS, INTERSECTIONS AND EACH SIDE OF WALL OPENINGS.
4. PROVIDE CONT HORIZ JT REINF @ 16" O.C. MAX. SEE MASONRY NOTES FOR ADDITIONAL REQUIREMENTS.

0'-0" (REF) = 99.5' (ACTUAL) T/SLAB (TYP UNO)
-2'-0" T/FTG (TYP UNO)

SCALE 1/8"=1'-0"

1' 4' 12'
0 2' 8' 24'

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FLORIDA REGISTRATION NUMBERS
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GENERAL NOTES

THEODORE G. FERY
FLORIDA REGISTRATION AR0007939

KEYED NOTES

STEVEN L. SHELT, PE 0036171

NO.:	DESCRIPTION:	DATE:
-	-	----/--/--

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4200 JOHN YOUNG PKWY
ORLANDO, FL 32839

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DISTRICT 5 COMMISSIONER	TED EDWARDS
DISTRICT 6 COMMISSIONER	TIFFANY MOORE RUSSELL

SLAB/FOUNDATION & ROOF FRAMING PLANS

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CONSULTING STRUCTURAL ENGINEERS
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WINTER PARK, FLORIDA 32789
EB #767

DRAWN BY:	RBJ
CHECKED BY:	KCB
SCALE:	1/8"
START DATE:	06.12.2013
PROJECT NO.:	3764A-00
SHEET NO.:	S-101

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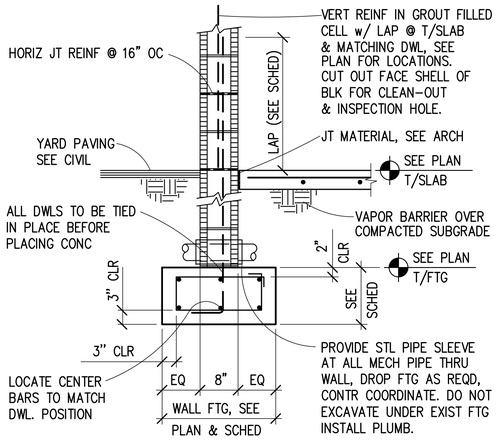
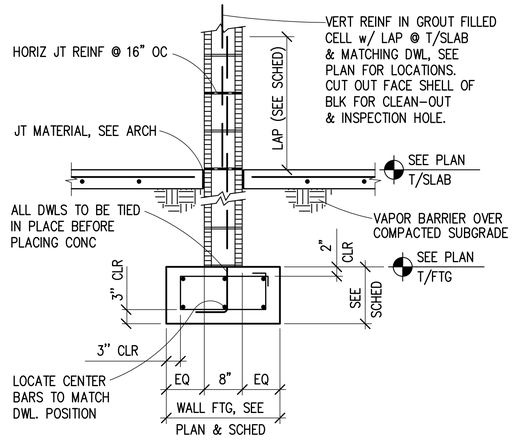


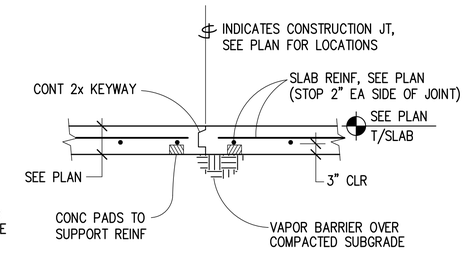
1 S501 EXT BLK WALL AT FTG

3/4" = 1'-0"



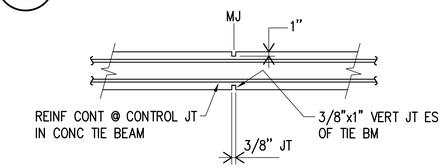
2 S501 INT BLK WALL AT FTG

3/4" = 1'-0"

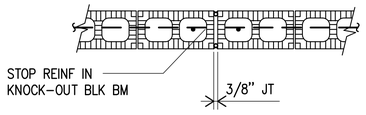


3 S501 SLAB CONST/CONTROL JT DET

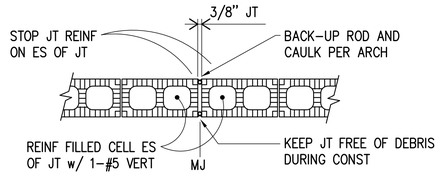
3/4" = 1'-0"



AT TIE BM/BM



AT LINTEL BLK BM

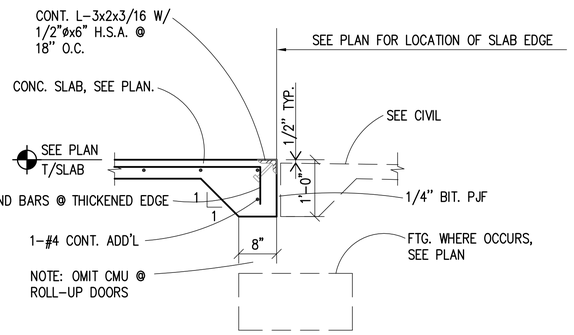


AT FOUNDATION

NOTE: 30'-0" OC (MAX) TYP-UNO

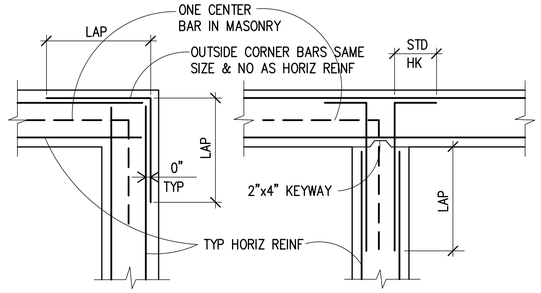
7 S501 MASONRY JOINT (MJ) DET

3/4" = 1'-0"



4 S501 SLAB EGE @ ROLL-UP DOOR / DRIVE THRU

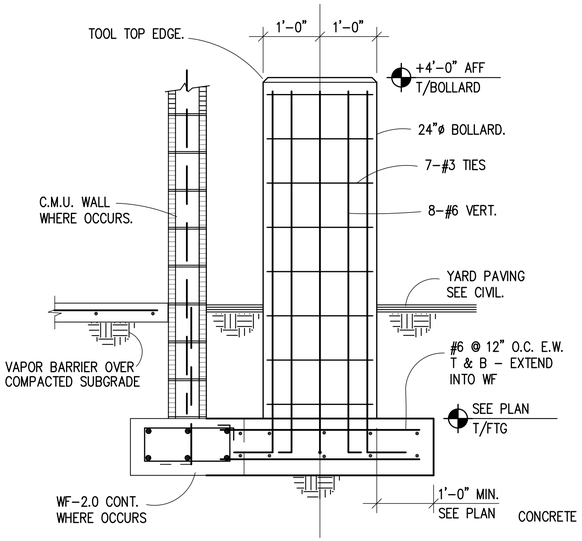
3/4" = 1'-0"



CORNERS INTERSECTIONS

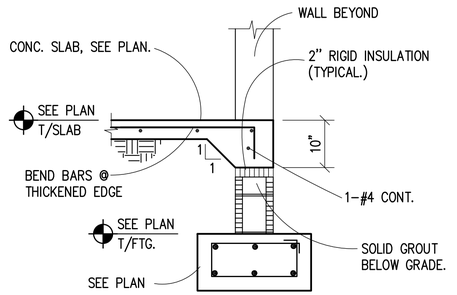
8 S501 TYP HORIZ REINF IN BMS, WALLS AND FTGS

3/4" = 1'-0"



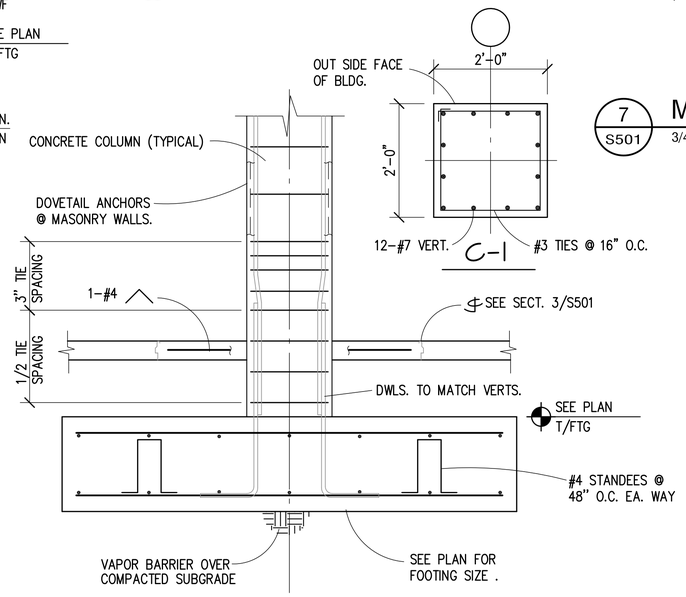
5 S501 BOLLARD DETAIL

3/4" = 1'-0"



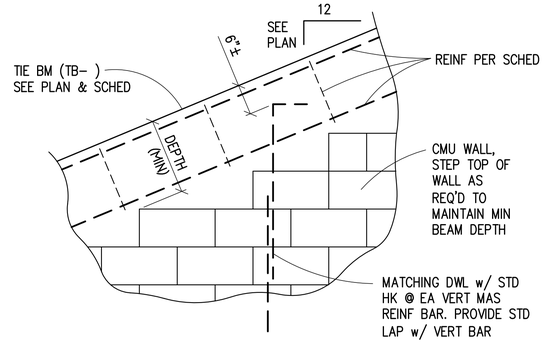
6 S501 SLAB @ EXTERIOR OPENING

3/4" = 1'-0"



9 S501 TYP COLUMN/FOOTING DETAIL

3/4" = 1'-0"



10 S501 TYP SLOPING TIE BEAM DET

3/4" = 1'-0"

0 1' 2' 4'

3/4" = 1'-0"

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FLORIDA REGISTRATION NUMBERS
#AA-C000148 / # IB-0000809

GENERAL NOTES

THEODORE G. FERY
FLORIDA REGISTRATION AR0007939

KEYED NOTES

STEVEN L. SHELT, PE 0036171

NO.:	DESCRIPTION:	DATE:
1	OWNER COMMENTS	01/16/2014
-	-	----/--/--

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ORLANDO, FL 32839

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DISTRICT 6 COMMISSIONER	TIFFANY MOORE RUSSELL

SECTIONS & DETAILS

DRAWN BY:	RBJ
CHECKED BY:	KCB
SCALE:	3/4"
START DATE:	06.12.2013
PROJECT NO.:	3764A-00

SHEET NO.: **S-501**

12047

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CONSULTING STRUCTURAL ENGINEERS
1280 PALMETTO AVENUE (407)628-5282
WINTER PARK, FLORIDA 32789
EB #767

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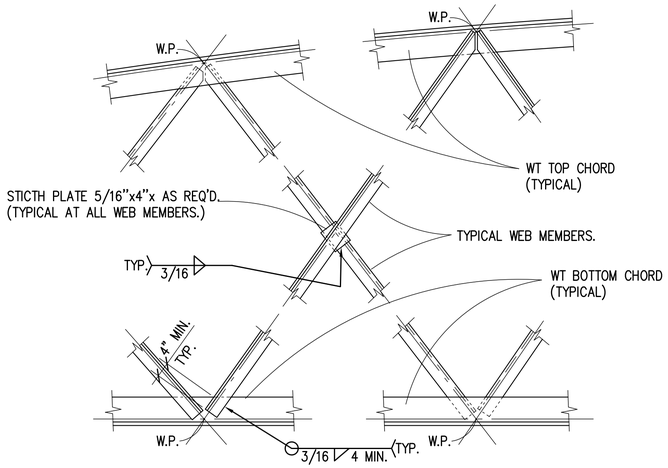
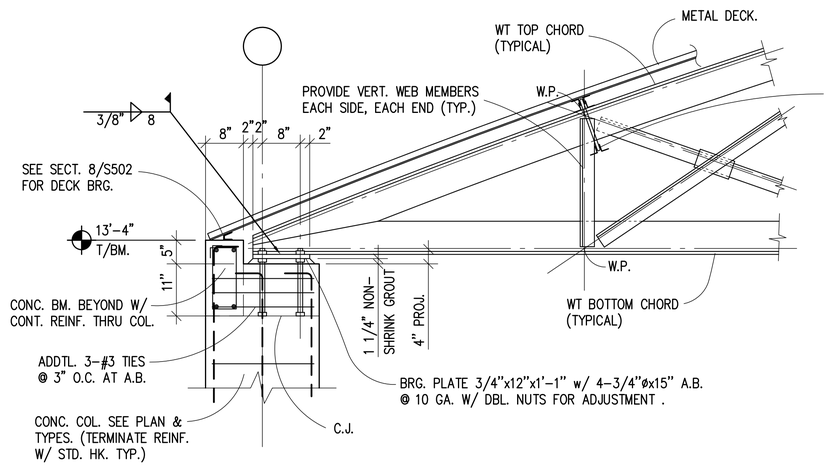


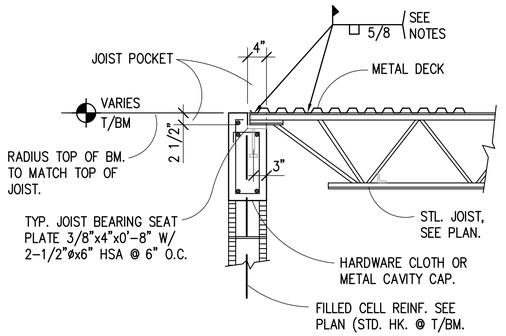
1 S502 TYP TRUSS CONN DETAIL

3/4" = 1'-0"



2 S502 TRUSS T-1 BEARING DET

3/4" = 1'-0"



3 S502 JOIST BRG @ CMU WALL

3/4" = 1'-0"

4 S502 TRUSS TYPE "T-1"

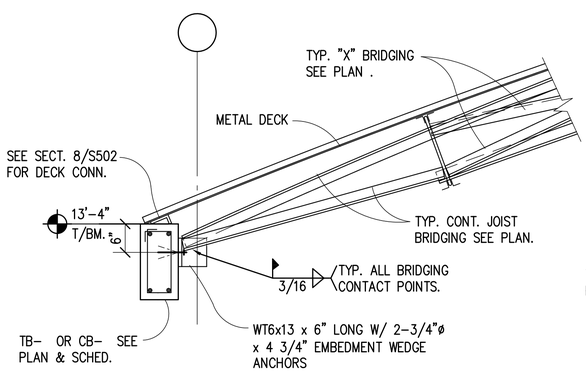
1/4" = 1'-0"

5 S502 DECK BRG @ CMU WALL

3/4" = 1'-0"

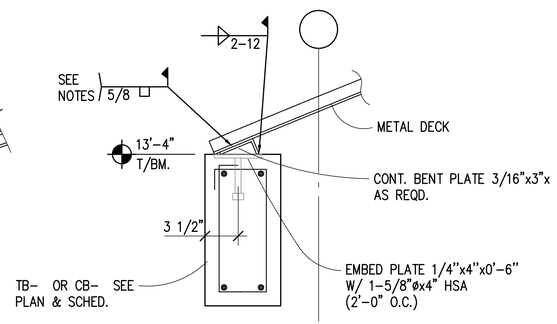
6 S502 TYP JOIST BRG @ TRUSS

3/4" = 1'-0"



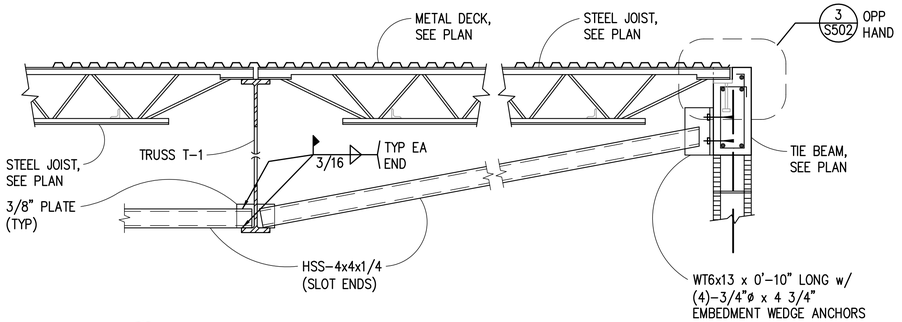
7 S502 TYP BRIDGING CONN

3/4" = 1'-0"



8 S502 TYP DECK CONNECTION

1 1/2" = 1'-0"



9 S502 TRUSS BOTTOM CHORD BRACE

3/4" = 1'-0"

0 1' 2' 4'

3/4" = 1'-0"

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1	OWNER COMMENTS	01/16/2014
-	-	----/--/--

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SECTIONS & DETAILS

DRAWN BY:	RBJ
CHECKED BY:	KCB
SCALE:	3/4"
START DATE:	06.12.2013
PROJECT NO.:	3764A-00

SHEET NO.: **S-502**

FILE: \ACBIGBRO\Drawings\DWGS\12047_12047-S-502 - Bldg 5a.dwg
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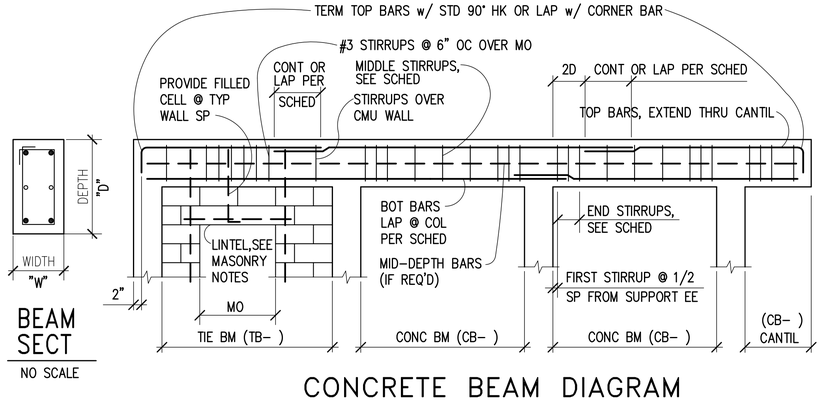


BEAM SECT

NO SCALE

CONCRETE BEAM DIAGRAM

NO SCALE SEE BM SCHED FOR BAR SIZES & SP

CONCRETE BEAM SCHEDULE

MARK	SIZE (W x D)	REINFORCING			STIRRUPS			REMARKS
		BOT BARS	TOP BARS	MID BARS (EF)	SIZE	# STIRRUPS @ SP OC EA END	# STIRRUPS @ SP OC MIDDLE	
CB–1	8x16	2–#6	2–#6	–	#3	12"	12"	
CB–2	8x24	2–#6	2–#6	1–#5	#3	10"	20"	
TB–1	8x16	2–#5	2–#5	–	#3	SEE DIAG	–	
TB–2	8x21	2–#6	2–#6	1–#6	#3	SEE DIAG	–	

MIN LAP SPLICE LENGTH SCHEDULE

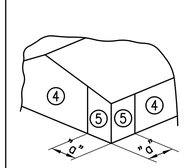
BAR TYPE	BAR SIZE								
	#3	#4	#5	#6	#7	#8	#9	#10	#11
FOOTINGS	16"	18"	22"	26"	38"	43"	49"	55"	61"
COLUMN	–	–	22"	26"	38"	45"	56"	69"	83"
WALLS	16"	18"	22"	26"	43"	54"	66"	81"	–
SLABS	16"	22"	32"	43"	69"	86"	–	–	–
BEAMS (TOP)	–	–	28"	34"	49"	59"	73"	90"	107"
BEAMS (MID & BOT)	–	–	22"	26"	38"	45"	56"	69"	83"
STIRRUPS	16"	18"	22"	26"	–	–	–	–	–
FILLED CELLS	SEE MASONRY NOTES								

FOOTING SCHEDULE

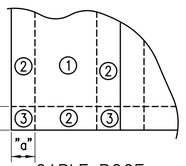
MARK	SIZE	REINFORCEMENT	REMARKS
	WIDTH x LENGTH x DEPTH		
WF–2.0	2'–0"xCONTx12"	3–#4 CONT T&B #3 TIES @ 48" OC	
F5.0	5'–0"x5'–0"x16"	5–#6 EW T&B	
F9.0x7.0	9'–0"x7'–0"x20"	7–#7 LW T&B 9–#7 SW T&B	

WIND PRESSURE FOR COMPONENTS CLADDING

CODE: ASCE 7–10
DESIGN WIND SPEED: 136 MPH
RISK CATEGORY: II
EXPOSURE: C
INTERNAL PRESSURE COEFFICIENT: +/–0.55



WALLS



GABLE ROOF

TRIB AREA (SQ FT)	ZONE	WIND PRESSURE AND SUCTION (PSF)	
		PRESSURE	SUCTION
10	4	55.2	–58.8
50	4	50.8	–54.4
500	4	44.5	–48.1
10	5	55.2	–69.5
50	5	50.8	–60.7
500	5	44.5	–48.1
10	1	37.4	–51.7
100	1	30.3	–48.1
10	2	37.4	–80.2
100	2	30.3	–62.4
10	3	37.4	–112.3
100	3	30.3	–90.9
10	2H	–	–78.4
10	3H	–	–131.9

NOTES:
1. a = 5 FT.
2. THE WIND DIRECTIONALITY FACTOR (kd) IS INCLUDED IN THE CALCULATION OF THESE PRESSURES.
3. H DENOTES OVERHANG VALUES.

0 1' 2' 4'

3/4" = 1'–0"

12047

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CONSTRUCTION DOCUMENTS - MARCH 10, 2014

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FLORIDA REGISTRATION NUMBERS
#AA-C000148 / # IB-0000809

GENERAL NOTES

THEODORE G. FERY
FLORIDA REGISTRATION AR0007939

KEYED NOTES

STEVEN L. SHELT, PE 0036171

NO.:	DESCRIPTION:	DATE:
-	-	----/--/--

ORANGE COUNTY STORM-WATER MANAGEMENT BUILDING 5A

4200 JOHN YOUNG PKWY
ORLANDO, FL 32839

LIST OF BOARD MEMBERS:

ORANGE COUNTY MAYOR	TERESA JACOBS
DISTRICT 1 COMMISSIONER	S. SCOTT BOYD
DISTRICT 2 COMMISSIONER	FRED BRUMMER
DISTRICT 3 COMMISSIONER	PETE CLARKE
DISTRICT 4 COMMISSIONER	JENNIFER THOMPSON
DISTRICT 5 COMMISSIONER	TED EDWARDS
DISTRICT 6 COMMISSIONER	TIFFANY MOORE RUSSELL

SECTIONS & DETAILS

DRAWN BY:	RBJ
CHECKED BY:	KCB
SCALE:	3/4"
START DATE:	06.12.2013
PROJECT NO.:	3764A-00
SHEET NO.:	S-601

FILE: \\ACBIGBRO\Drawings\DWGS\12047\ 12047-S-601 - Bldg 5a.dwg
Plot Date: 3/4/2014 6:48:21 AM